

Load effect analysis of semi-submersible floating wind turbine tower, considering sum-frequency wave excitation

Haozhe Bai^{1,2*}, Shuaishuai Wang², Torgeir Moan², Kun Xu¹, Min Zhang¹ and Huajun Li¹

¹ College of Engineering, Ocean University of China, Qingdao, China

² Department of Marine Technology, Norwegian University of Science and Technology, Trondheim, Norway



Introduction

Previous Experimental Results

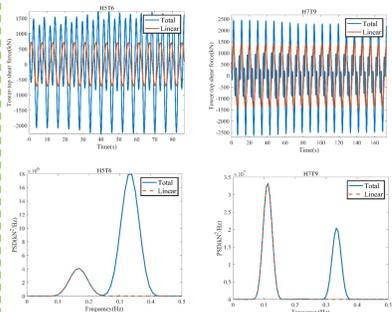


Fig 1. What are WIHF dynamic responses of semi-submersible FWT

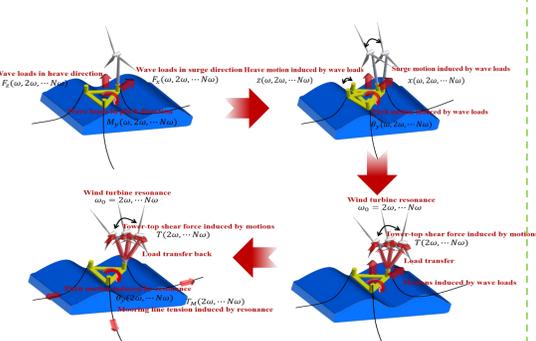
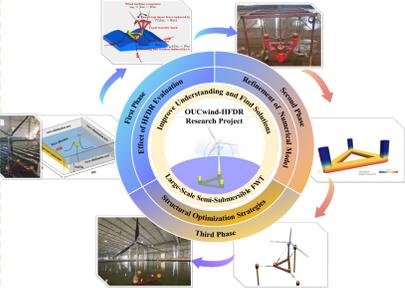


Fig 2. The mechanism of WIHF dynamic responses of semi-submersible FWT

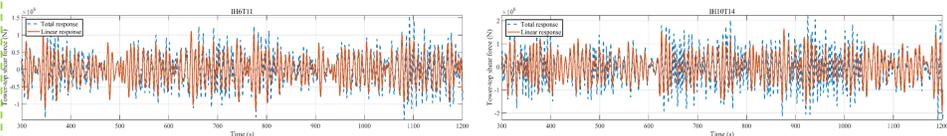
Research project: OUCwind-HFDR



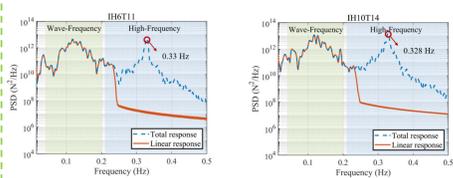
- Most adopted type: **semi-submersible FWT**
- The **sum-frequency wave load effects on semi-submersible FWT** have attracted minimal focus
- A comprehensive and progressive research project “**OUCwind-HFDR**”
- Investigates the **WIHF structural responses of a semi-submersible FWT under various environmental conditions**

Results and Discussion

The experimental WIHF structural load under irregular wave condition

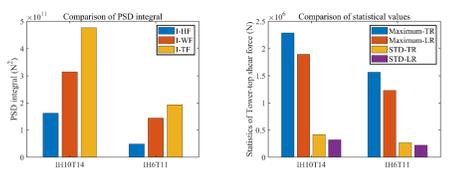


(a) The time series of tower-top shear force under IH6T11 (left) and IH10T14 (right)



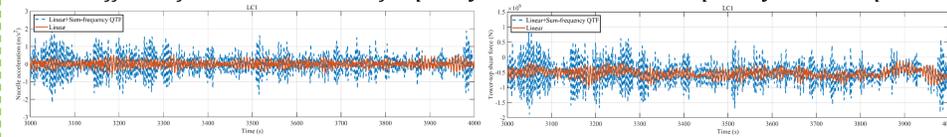
(b) The PSD of tower-top shear force under IH6T11 (left) and IH10T14 (right)

- The high-frequency peak frequencies are 0.33 Hz and 0.328 Hz, respectively, which are remarkably close to each other.
- A proportional coefficient α is defined by I_{HF}/I_{TF} . The α s under IH6T11 and IH10T14 reach to 0.25 and 0.34, respectively. Additionally, the maximum and STD of tower-top shear force are respectively underestimated by linear response by 21.57 % and 17.19 % under IH6T11, and 17.29 % and 23.39 % under IH10T14.

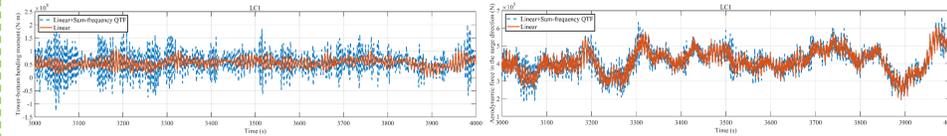


(c) The PSD integral (left) and statistics (right) of tower-top shear force

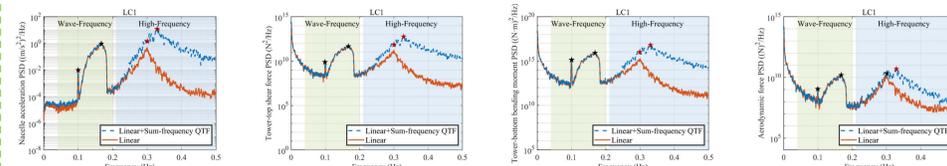
The effects of second-order sum-frequency wave load on the coupled dynamic responses



(a) The time series of nacelle acceleration (left) and tower-top shear force (right)



(b) The time series of tower-bottom bending moment (left) and aerodynamic force (right)

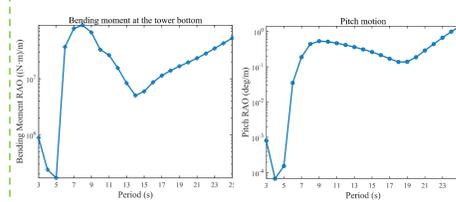


(c) The PSDs of nacelle acceleration, tower-top shear force, tower-bottom bending moment, and aerodynamic force

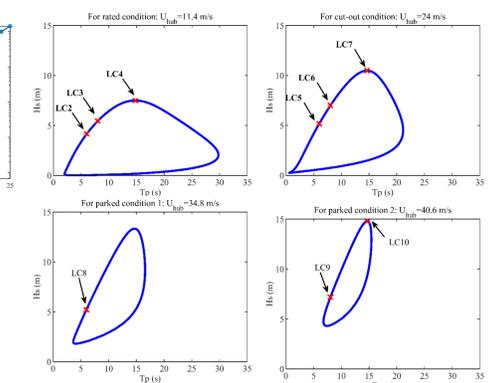
- It could be observed that there are two peaks in the wave-frequency region. The left one equal to 0.1 Hz corresponds to the rotor rotational frequency (1P), and the right one corresponds to the wave peak period.
- Two peaks in the high-frequency region both have considerable disparities between NHM and LHM. Among these two peaks, the left one equal to 0.3 Hz is associated with the blade passing frequency (3P), and the right one close to 0.33 Hz is relevant with the natural frequency of the wind turbine with an elastic boundary.

Environmental Condition Design

RAO analysis



Contour figures to determine wave conditions



- **Wind conditions:** four typical conditions including cut-in, rated, cut-out, and parked conditions.

Wave conditions:

- **Cut-in condition:** significant wave height of 3 m and a peak period of 6 s is selected
- **Rated, cut-out, and parked conditions:**
 1. special wave period of 6s
 2. select based on the RAO analysis
 3. largest significant wave height

Ten typical wind and wave combined conditions

Condition	Load cases	U_{hub} (m/s)	TI	Hs (m)	Tp (s)
Cut-in condition	LC1	5	23.04%	3.00	6.00
	LC2	11.4	13.41%	4.15	6.00
Rated condition	LC3	11.4	13.41%	5.44	8.00
	LC4	11.4	13.41%	7.50	14.78
Cut-out condition	LC5	25	13.23%	5.17	6.00
	LC6	25	13.23%	7.00	8.00
Parked Condition	LC7	25	13.23%	10.48	14.62
	LC8	34.8	14.77%	5.22	6.00
	LC9	40.6	14.77%	7.15	8.00
	LC10	40.6	15.61%	14.78	14.67

Conclusions

ULS check for DTU 10-MW reference wind turbine

- Most checks of LHM pass except for the column buckling check under particularly severe conditions, and the rated condition is the most unfavourable condition.
- All checks of NHM fail to pass under rated, cut-out, and parked conditions. Moreover, as the wave conditions become more severe, the check coefficients of all checks are farther away from the safety criterion.

Load condition	Hydrodynamic model	Yielding Check coefficient	Buckling check coefficient for shell	Buckling check coefficient for column
Cut-in LC1	LHM	6.1784 (pass)	5.9084 (pass)	0.3782 (pass)
	NHM	2.2187 (pass)	1.5127 (pass)	0.8682 (pass)
Rated LC3	LHM	1.8466 (pass)	1.7610 (pass)	0.8531 (pass)
	NHM	0.7285 (fail)	0.6023 (fail)	2.2439 (fail)
Cut-out LC6	LHM	3.2189 (pass)	1.9339 (pass)	0.5558 (pass)
	NHM	0.4299 (fail)	0.2596 (fail)	3.6099 (fail)
Parked LC10	LHM	2.0540 (pass)	1.9596 (pass)	1.2262 (fail)
	NHM	0.2820 (fail)	0.1882 (fail)	5.7328 (fail)

Remarkable conclusions

- **Experimental results show that WIHF structural loads are significant to the total structural loads under irregular wave conditions, and the extent of impact is positively correlated with the increase in the severity of sea conditions.**
- **The second-order high-frequency wave loads could amplify the high-frequency structural responses caused by the 3P and resonance effects.**
- **The second-order high-frequency wave loads could increase the aerodynamic force through increasing the components of the natural frequency of the wind turbine, but it has minor effects on the components of the rotational effects of rotor.**
- **Considering the second-order sum-frequency wave load effects, the most dangerous condition for the ULS check for the critical section of DTU-10-MW reference wind turbine is shifted to the parked condition from the rated condition.**

Acknowledgement and Contact

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Contact

- **Email:** bhz@stu.ouc.edu.cn



Haozhe Bai
 PhD Candidate at Ocean University of China · Position · Ocean University of China
 China | Website



⁹⁹ Exchanged PhD student at NTNU in Norway