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Optimization of Offshore Wind Turbine Support Structures Using Analytical Gradient-Based Method

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Outline

- Background
- Optimal design problem
- Integrated optimization framework
- Case study: OC4 jacket substructure
- Conclusions & Future work

Background

- Optimization of offshore wind turbine support structures
 - Highly constrained
 - Non-convex
 - Non-linear
 - Dynamic problem
- Sensitivity analysis
 - Finite difference approximation
 - Inefficient & numerical error

Non-linear dynamic
response constrained
structural optimization

Optimal design problem

Find \mathbf{b} to minimize $f(\mathbf{b})$

$$f = \sum_{n=1}^N \rho A_n L_n \text{ where } A_n = \pi(D_n t_n - t_n^2)$$

Objective func

subject to $g_i(\mathbf{b}, \mathbf{z}(t_j), \dot{\mathbf{z}}(t_j), \ddot{\mathbf{z}}(t_j), t_j) \leq 0$ and $\mathbf{M}\ddot{\mathbf{z}}(t_j) + \mathbf{C}\dot{\mathbf{z}}(t_j) + \mathbf{K}\mathbf{z}(t_j) - \mathbf{f}(t_j) = 0$ where $i=1, \dots, p, j=1, \dots, q$

$$g_1 = \mathbf{b}_{\min} \leq \mathbf{b} \leq \mathbf{b}_{\max}$$

Sizing

$$g_2 = \mathbf{A}_{ineq} \mathbf{b} - \mathbf{c}_{ineq} \leq 0$$

Eigenfrequency

$$g_3 = 0.222 \text{ Hz} \leq f_1 \leq 0.311 \text{ Hz}$$

Tension, bending

$$g_{4a} = \left(N_{t,Sd} / N_{t,Rd}\right)^{1.75} + \left(M_{y,Sd}^2 + M_{z,Sd}^2\right)^{0.5} / M_{Rd} - 1.0 \leq 0$$

Buckling

$$g_{5a} = N_{c,Sd} / N_{c,Rd} + \left\{ \left[C_{my} M_{y,Sd} / \left(1 - N_{c,Sd} / N_{Ey}\right) \right]^2 + \left[C_{mz} M_{z,Sd} / \left(1 - N_{c,Sd} / N_{Ez}\right) \right]^2 \right\}^{0.5} / M_{Rd} - 1.0 \leq 0$$

Compression

$$g_{6a} = N_{c,Sd} / N_{cl,Rd} + \left(M_{y,Sd}^2 + M_{z,Sd}^2\right)^{0.5} / M_{Rd} - 1.0 \leq 0$$

Extreme load (beam)

$$g_7 = \left(\sigma_{c,Sd} - 0.5 f_{he} / \gamma_m\right) / \left(f_{cle} / \gamma_m - 0.5 f_{he} / \gamma_m\right) + \left(\sigma_{p,Sd} \gamma_m / f_{he}\right)^2 - 1.0 \leq 0$$

Shear, bending & torsion

$$g_{8a} = M_{Sd} / \left[M_{Red,Rd} \left(1.4 - V_{Sd} / V_{Rd}\right)^{0.5} \right] - 1.0 \leq 0 \text{ for } V_{Sd} / V_{Rd} \geq 0.4$$

Hoop buckling

$$g_9 = \sigma_{p,Sd} / f_{h,Rd} - 1.0 \leq 0$$

Extreme load (joint)

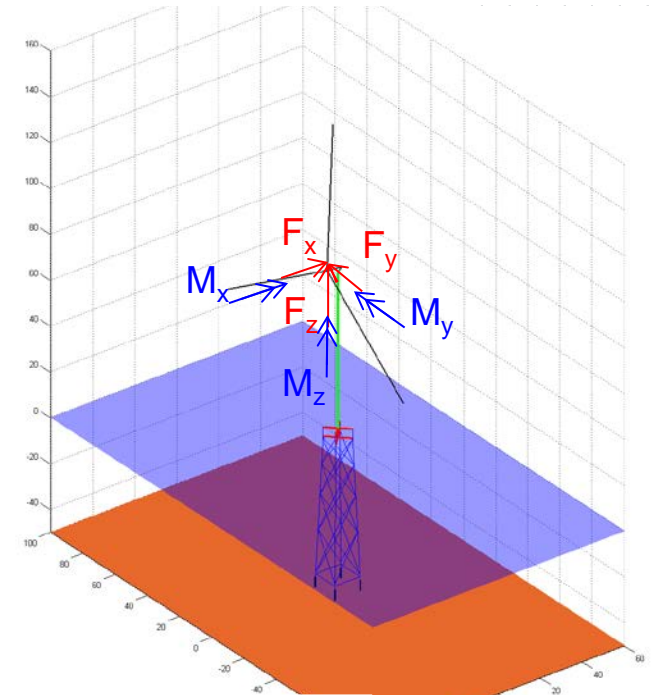
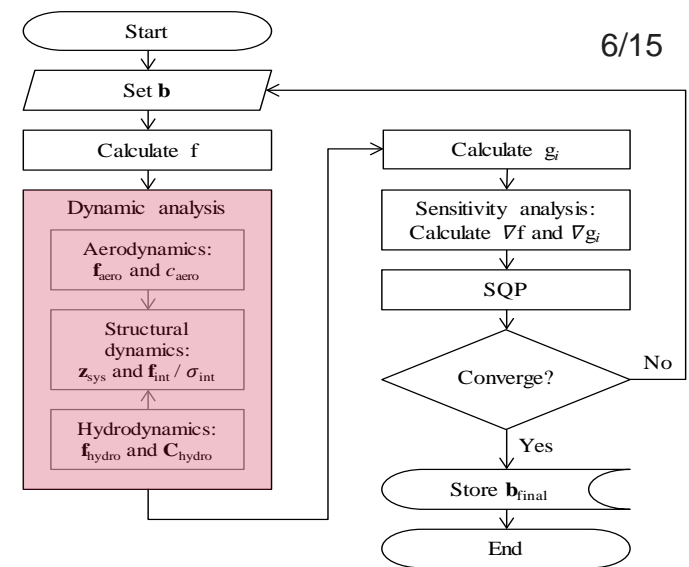
$$g_{10} = N_{Sd} / N_{Rdj} + \left(M_{y,Sd} / M_{y,Rdj}\right)^2 + M_{z,Sd} / M_{z,Rdj} - 1.0 \leq 0$$

Fatigue load

$$g_{11} = \sum_{i=1}^l \frac{1}{\bar{a}\eta} \left(\frac{t}{t_{ref}}\right)^{mk} n_i (\Delta\sigma_{i,HSS})^m - 1.0 \leq 0$$

Optimization framework

- Time domain dynamic analysis
 - Structural dynamic
 - Euler-Bernoulli beam
 - Rayleigh damping
 - Newmark-beta integration
 - Hydrostatic & hydrodynamic
 - Buoyancy
 - Internal water mass; marine growth
 - Morison formula
 - Aerodynamic
 - Decoupled model
 - Aerodynamic external force
 - Aerodynamic damping



Optimization framework

- Analytical sensitivity analysis

- Direct differentiation method

$$\nabla f = \frac{\partial f}{\partial \mathbf{b}} \quad \nabla g_i = \frac{\partial g_i}{\partial \mathbf{b}} + \frac{\partial g_i}{\partial \mathbf{z}} \frac{d\mathbf{z}}{d\mathbf{b}}$$

- Extreme load constraints

$$\frac{d\mathbf{z}_{sta}}{d\mathbf{b}} = \mathbf{K}^{-1} \left(\frac{d\mathbf{f}_{sta}}{d\mathbf{b}} - \frac{d\mathbf{K}}{d\mathbf{b}} \mathbf{z}_{sta} \right) \quad \mathbf{M} \frac{d^2}{dt^2} \left(\frac{d\mathbf{z}_{dyn}}{d\mathbf{b}} \right) + \mathbf{C} \frac{d}{dt} \left(\frac{d\mathbf{z}_{dyn}}{d\mathbf{b}} \right) + \mathbf{K} \left(\frac{d\mathbf{z}_{dyn}}{d\mathbf{b}} \right) = \frac{d\mathbf{f}_{dyn}}{d\mathbf{b}} - \frac{d\mathbf{M}}{d\mathbf{b}} \ddot{\mathbf{z}}_{dyn} - \frac{d\mathbf{C}}{d\mathbf{b}} \dot{\mathbf{z}}_{dyn} - \frac{d\mathbf{K}}{d\mathbf{b}} \mathbf{z}_{dyn}$$

- Fatigue load constraints

$$\nabla g_{11} = \sum_{i=1}^I \frac{mk}{\bar{a}\eta} \left(\frac{1}{t_{ref}} \right)^{mk} t^{mk-1} \frac{dt}{d\mathbf{b}} n_i (\Delta\sigma_{i,HSS})^m + \sum_{i=1}^I \frac{m}{\bar{a}\eta} \left(\frac{t}{t_{ref}} \right)^{mk} n_i (\Delta\sigma_{i,HSS})^{m-1} \frac{d\Delta\sigma_{i,HSS}}{d\mathbf{b}}$$

$$\frac{d\Delta\sigma_{i,HSS}}{d\mathbf{b}} = \begin{cases} \left. \frac{d\sigma_{i,HSS}}{d\mathbf{b}} \right|_{t_j=t_1} - \left. \frac{d\sigma_{i,HSS}}{d\mathbf{b}} \right|_{t_j=t_2} & \text{for } \sigma_{i,HSS}(t_1) > \sigma_{i,HSS}(t_2) \\ \left. \frac{d\sigma_{i,HSS}}{d\mathbf{b}} \right|_{t_j=t_2} - \left. \frac{d\sigma_{i,HSS}}{d\mathbf{b}} \right|_{t_j=t_1} & \text{for } \sigma_{i,HSS}(t_2) > \sigma_{i,HSS}(t_1) \end{cases}$$

$$\frac{d\sigma_{i,HSS}}{d\mathbf{b}} = c_{N_x} \left(\frac{dSCF_{N_x}}{d\mathbf{b}} \sigma_{i,N_x} + \frac{d\sigma_{i,N_x}}{d\mathbf{b}} SCF_{N_x} \right) + c_{M_y} \left(\frac{dSCF_{M_y}}{d\mathbf{b}} \sigma_{i,M_y} + \frac{d\sigma_{i,M_y}}{d\mathbf{b}} SCF_{M_y} \right) + c_{M_z} \left(\frac{dSCF_{M_z}}{d\mathbf{b}} \sigma_{i,M_z} + \frac{d\sigma_{i,M_z}}{d\mathbf{b}} SCF_{M_z} \right)$$

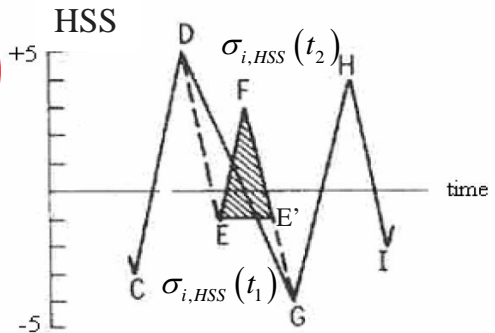
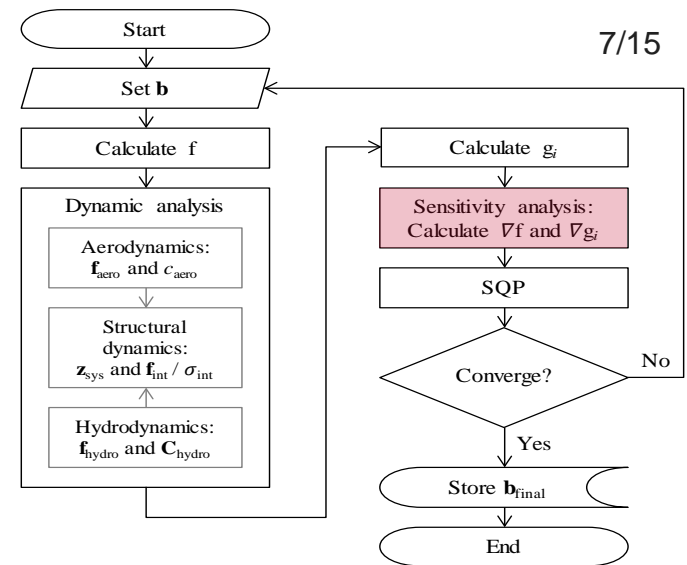
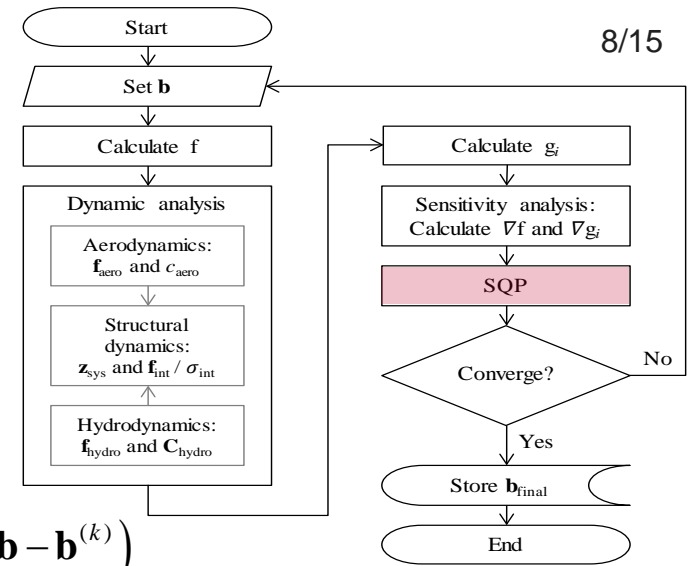


Fig. Rainflow counting

Optimization framework

- Sequential Quadratic Programming (SQP)
 - Reformulate as QP subproblem



$$\min f(\mathbf{b}^{(k)}) + (\nabla f(\mathbf{b}^{(k)}))^T (\mathbf{b} - \mathbf{b}^{(k)}) + \frac{1}{2} (\mathbf{b} - \mathbf{b}^{(k)})^T \mathbf{H}(\mathbf{b}^{(k)}) (\mathbf{b} - \mathbf{b}^{(k)})$$

$$\text{s.t. } g_i(\mathbf{b}^{(k)}) + (\nabla g_i(\mathbf{b}^{(k)}))^T (\mathbf{b} - \mathbf{b}^{(k)}) \leq 0, \quad i=1, \dots, p$$

- Hessian \mathbf{H} of Lagrangian function $L(\mathbf{b}, \lambda) = f(\mathbf{b}) + \sum_{i=1}^m \lambda_i g_i(\mathbf{b})$

$$\mathbf{H}^{(k+1)} = \mathbf{H}^{(k)} + \frac{\mathbf{q}^{(k)} (\mathbf{q}^{(k)})^T}{(\mathbf{q}^{(k)})^T \mathbf{s}^{(k)}} - \frac{\mathbf{H}^{(k)} \mathbf{s}^{(k)} (\mathbf{s}^{(k)})^T (\mathbf{H}^{(k)})^T}{(\mathbf{s}^{(k)})^T \mathbf{H}^{(k)} \mathbf{s}^{(k)}}$$

where $\mathbf{s}^{(k)} = \mathbf{b}^{(k+1)} - \mathbf{b}^{(k)}$

$$\mathbf{q}^{(k)} = \nabla L(\mathbf{b}^{(k+1)}, \lambda) - \nabla L(\mathbf{b}^{(k)}, \lambda)$$

Broyden-Fletcher-Goldfarb-Shanno (BFGS) approximation

- Matlab optimization toolbox

Case study- *Problem formulation*

- Design variables
 - OC4 jacket substructure
 - 22 variables (diameter & thickness)
 - Sizing constraints
 - $\mathbf{b}_{\min} = 33\% * \mathbf{b}_{\text{initial}}$ $\mathbf{b}_{\max} = 300\% * \mathbf{b}_{\text{initial}}$
 - $20 \leq D_{\text{brace}} / t_{\text{brace}} \leq 120$
 - $20 \leq D_{\text{leg}} / t_{\text{leg}} \leq 64$
 - $0.2 \leq D_{\text{brace}} / D_{\text{leg}} \leq 1.0$
 - $0.2 \leq t_{\text{brace}} / t_{\text{leg}} \leq 1.0$
 - Extreme load constraints
 - 1248 (beam); 104 (joints)
 - Fatigue load constraints
 - 208

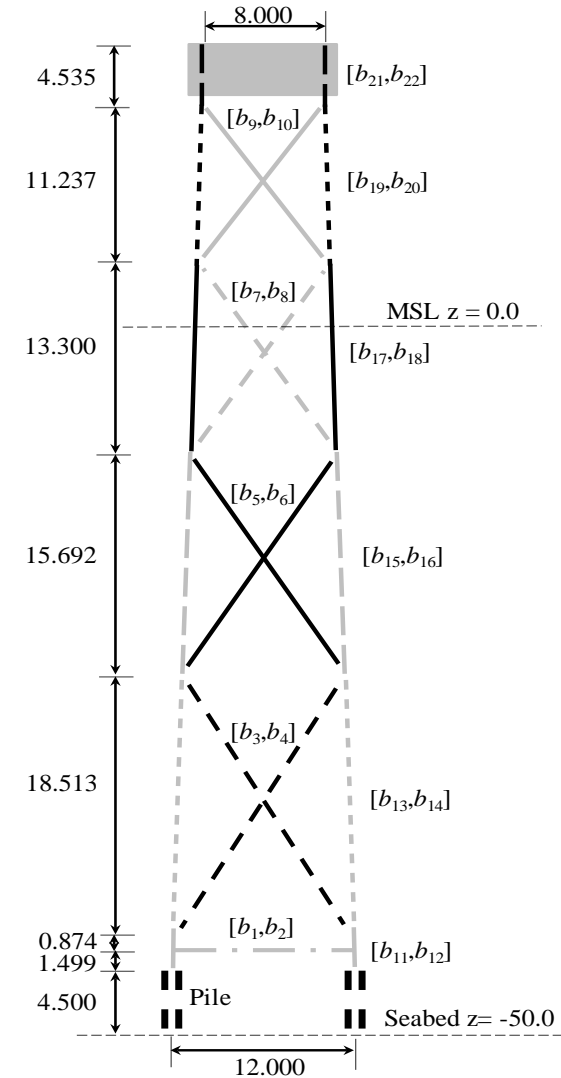


Fig. OC4 jacket substructure

Case study- *Problem formulation*

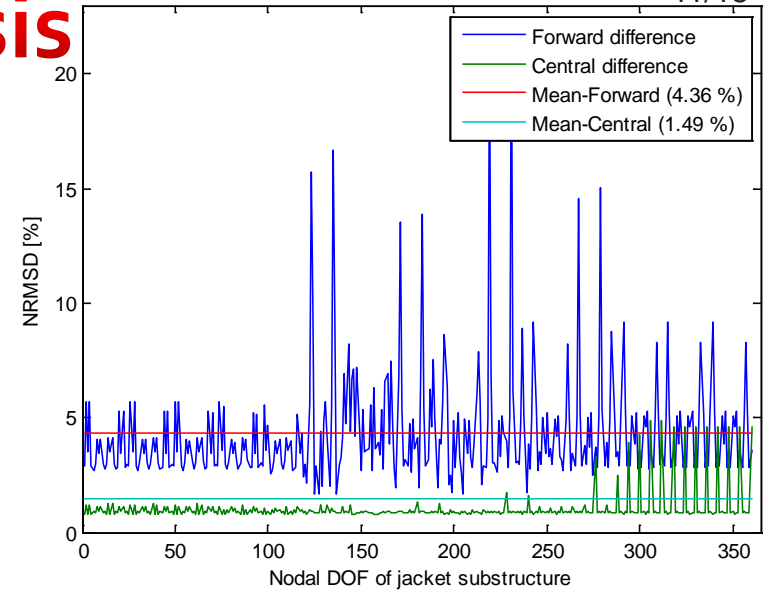
- Design load cases
 - Fatigue load case
 - Extreme load case

Load Case	Wind Conditions	Wave Conditions
FLS	NTM (Kaimal spectrum) $V_{\text{hub}} = 8.00 \text{ m/s}$ $TI = 10.00 \%$ $\alpha = 0.14$	NSS (JONSWAP spectrum) $H_s = 1.31 \text{ m}$ $T_p = 5.67 \text{ s}$ $\gamma = 1$
ULS	EWM (Kaimal spectrum) $V_{\text{hub}} = 42.73 \text{ m/s}$ $TI = 10.00 \%$ $\alpha = 0.11$	ESS (JONSWAP spectrum) $H_s = 9.40 \text{ m}$ $T_p = 13.70 \text{ s}$ $\gamma = 3.3$

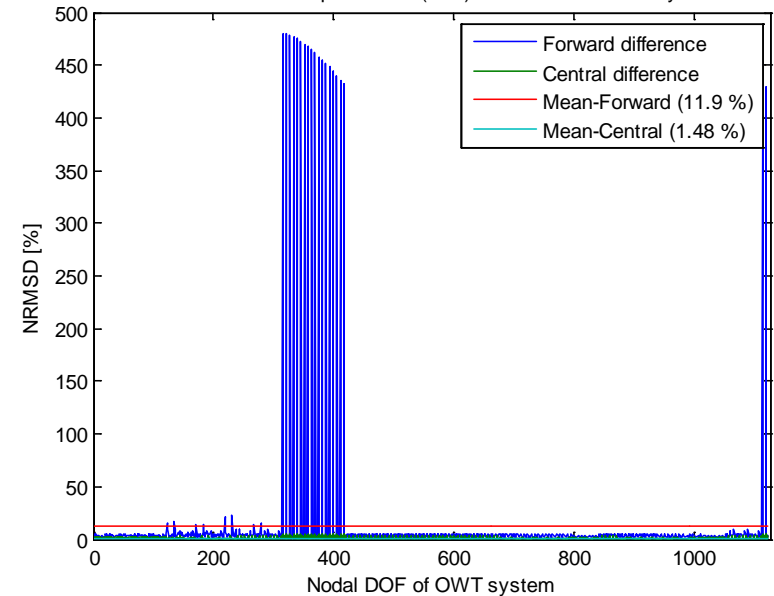
Results- Sensitivity analysis

- Sensitivity analysis
 - *Central vs Forward* FD
 - Integration *time step* (0.01 s & 0.025 s)
 - Design *variables* (braces & legs)
- Displacement (ULS & FLS)
 - *Central & DDM*
 - 2.5 % NRMSD (jacket)
 - *Forward & DDM*
 - 6.0 % NRMSD (jacket)
 - Large deviation for entire OWT

NRMSD of nodal displacement (FLS) derivatives for jacket 11/15



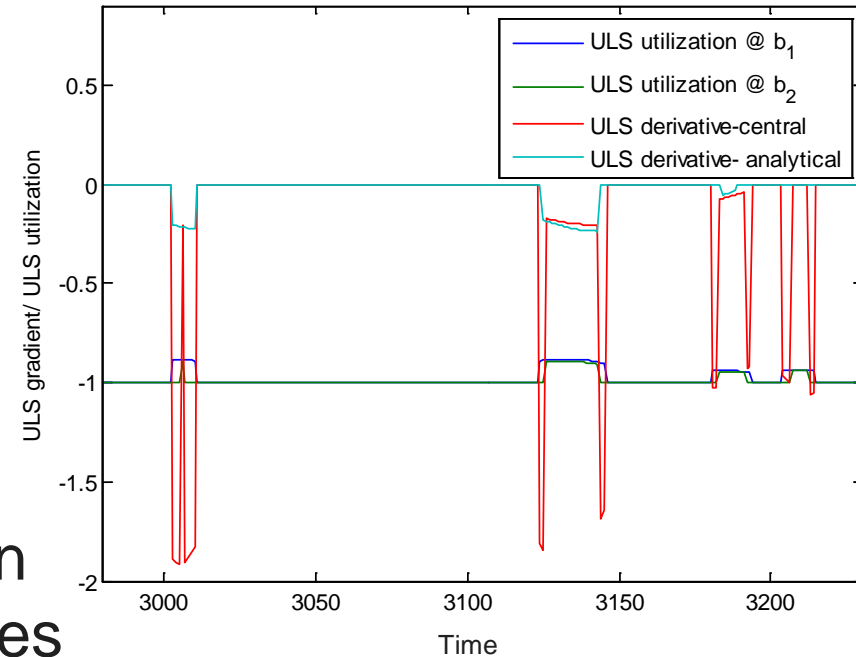
NRMSD of nodal displacement (FLS) derivatives for OWT system



Results- Sensitivity analysis

- Eigenfrequency
 - Small NRMDS
- Extreme load (beam)
 - Large NRMDS ~ 22 %
 - Numerical artifacts in FD
 - Discontinuity due to switch in compression & tension modes
- Extreme load constraints (joints)
 - Central & DDM: 2.5 %; Forward & DDM: 6 %
- Fatigue load constraints
 - Central & DDM: 5 %; Forward & DDM: 7 %
 - Influenced by other derivatives, e.g. SCF

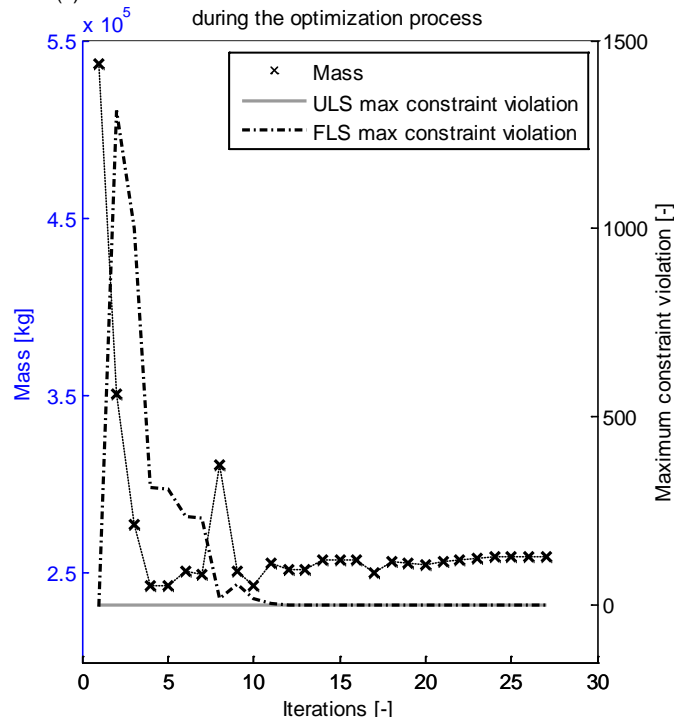
Comparison of ULS derivatives against b_{15} for beams between central difference and analytical



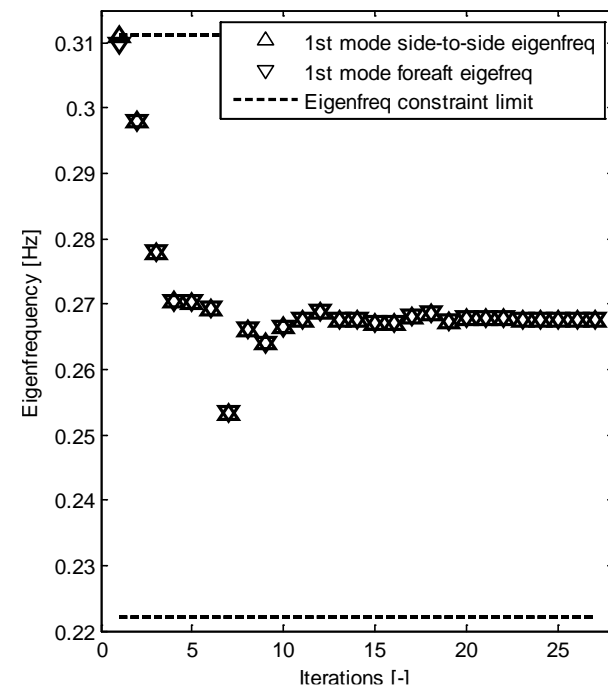
Results- Optimization

- Optimization
 - 52 % mass reduction
 - 27 iterations
 - Active constraints
- Eigen-frequency
 - Variation pattern
 - Converged to center of constraint limit

(a) Variation of Structural Mass and Max Constraint Violation during the optimization process



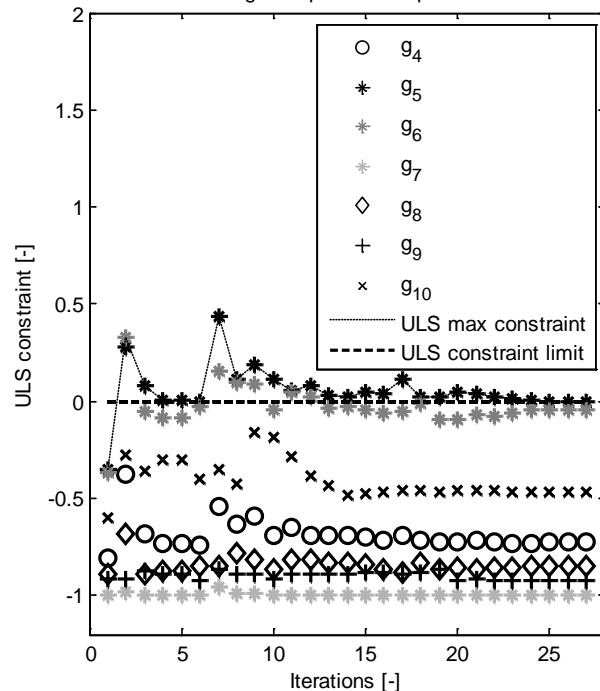
(b) Variation of First mode Eigenfrequencies during the optimization process



Results- Optimization

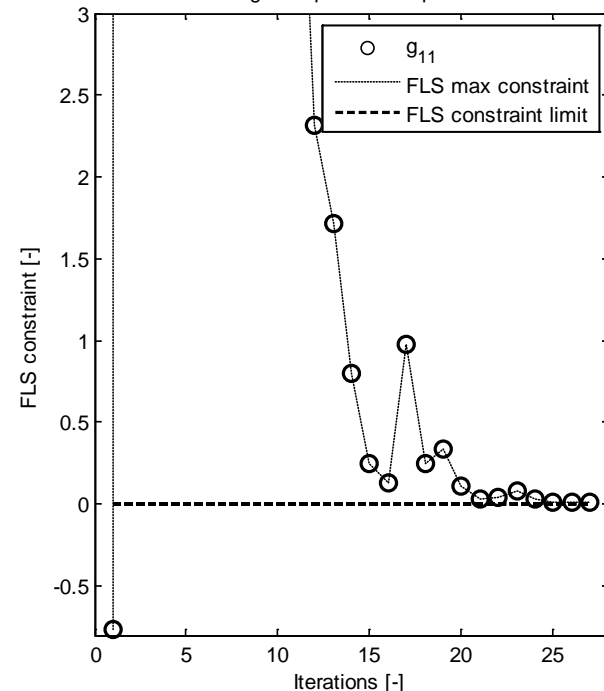
- Extreme load constraints
 - Buckling & compression
 - Others remained inactive

(c) Variation of Extreme Load Constraints during the optimization process



- Fatigue load constraints
 - Critical constraint

(d) Variation of Fatigue Load Constraints during the optimization process



Conclusions & Future work

- Successful key implementations:
 - Optimization in **time** domain, s.t. comprehensive **constraints**
 - Sensitivity analysis using **analytical DDM**
- Sensitivity analysis results
 - **Central** difference **matches well** with **DDM**
 - DDM is **twice more efficient**
 - DDM can **avoid numerical artifacts**
- Optimization results
 - Optimal design converges in **27 iterations**
 - Both **FLS & ULS** influence optimal design
 - **Buckling & compression** constraints are active
- Future work
 - Optimization s.t. complete fatigue load cases
 - Include geometrical/ topological variation

Thank you

Questions and Answers