

# Use of steel for towers of wind turbines and support structures

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# Overview



Background



State of the Art



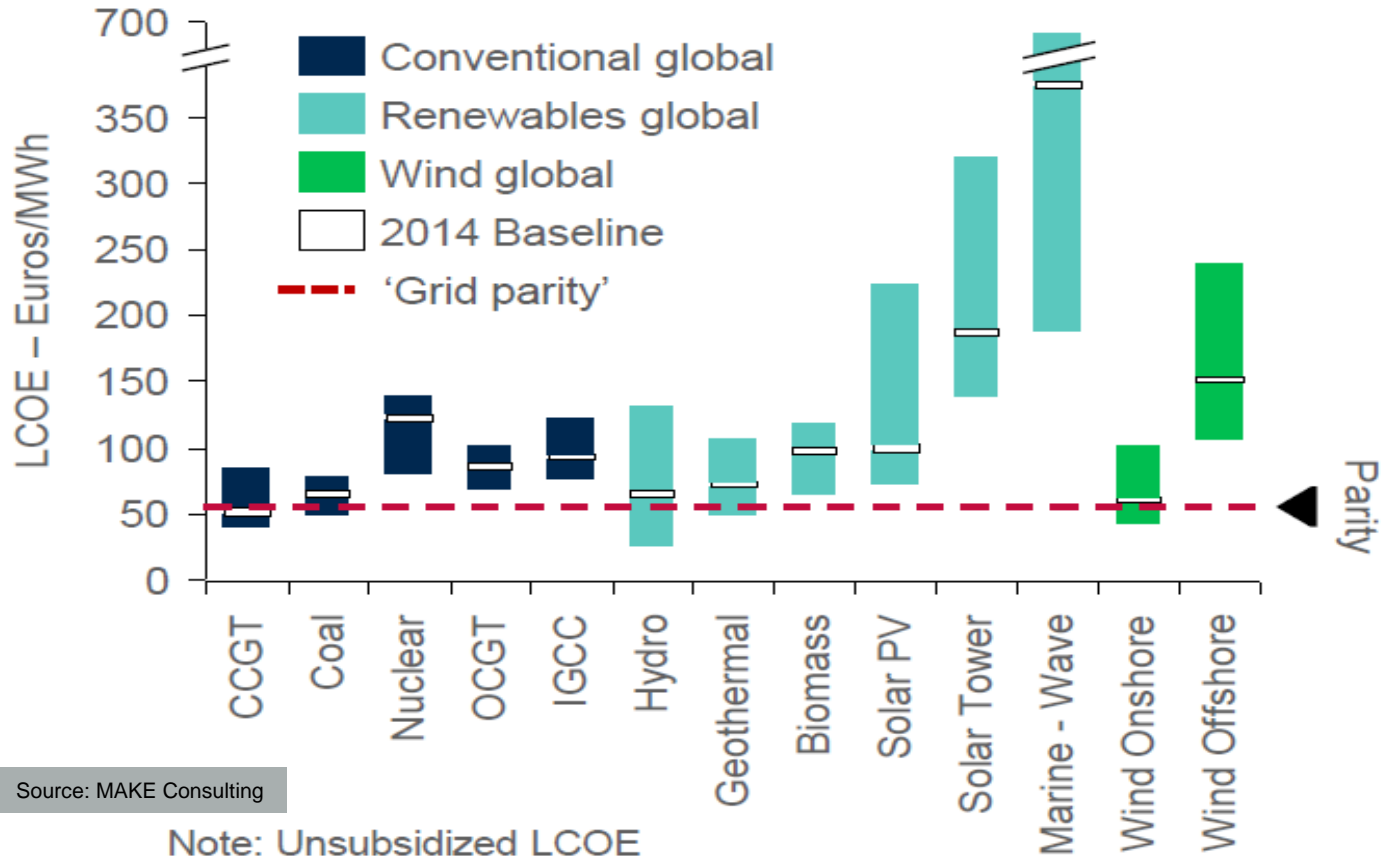
Problems and possibilities



Outlook

# Wind Energy and other energy sources

## 2014 LCOE – Global ranges and baselines



Source: MAKE Consulting

Note: Unsubsidized LCOE

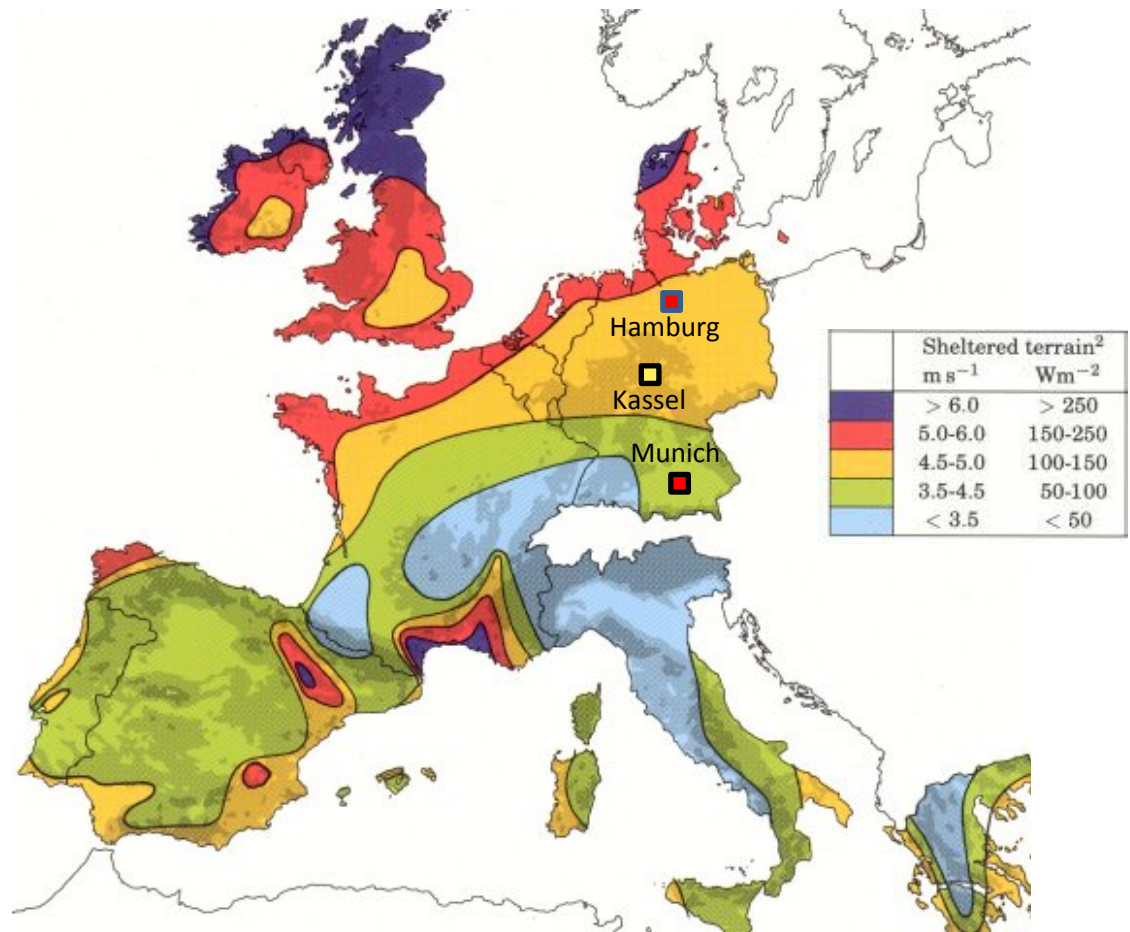


# How about inshore, instead of offshore?

$V_{av., 50 m}$  Hamburg: 5 m/s  
 $V_{av., 50 m}$  near the coast: 6 m/s),

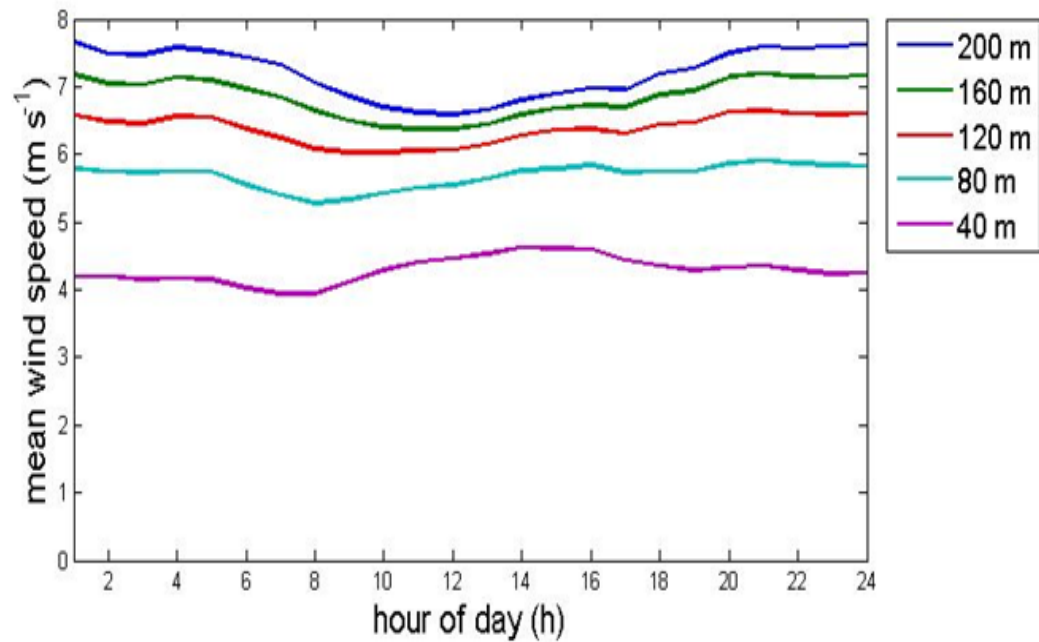
$V_{av., 50 m}$  Munich: 4 m/s

$E_{wind} \sim v^3$   
SO  $E_{Coast} = 2E_{Hamburg} = 4E_{Munich}$



Source: the European Wind Atlas





# Too little wind inshore?



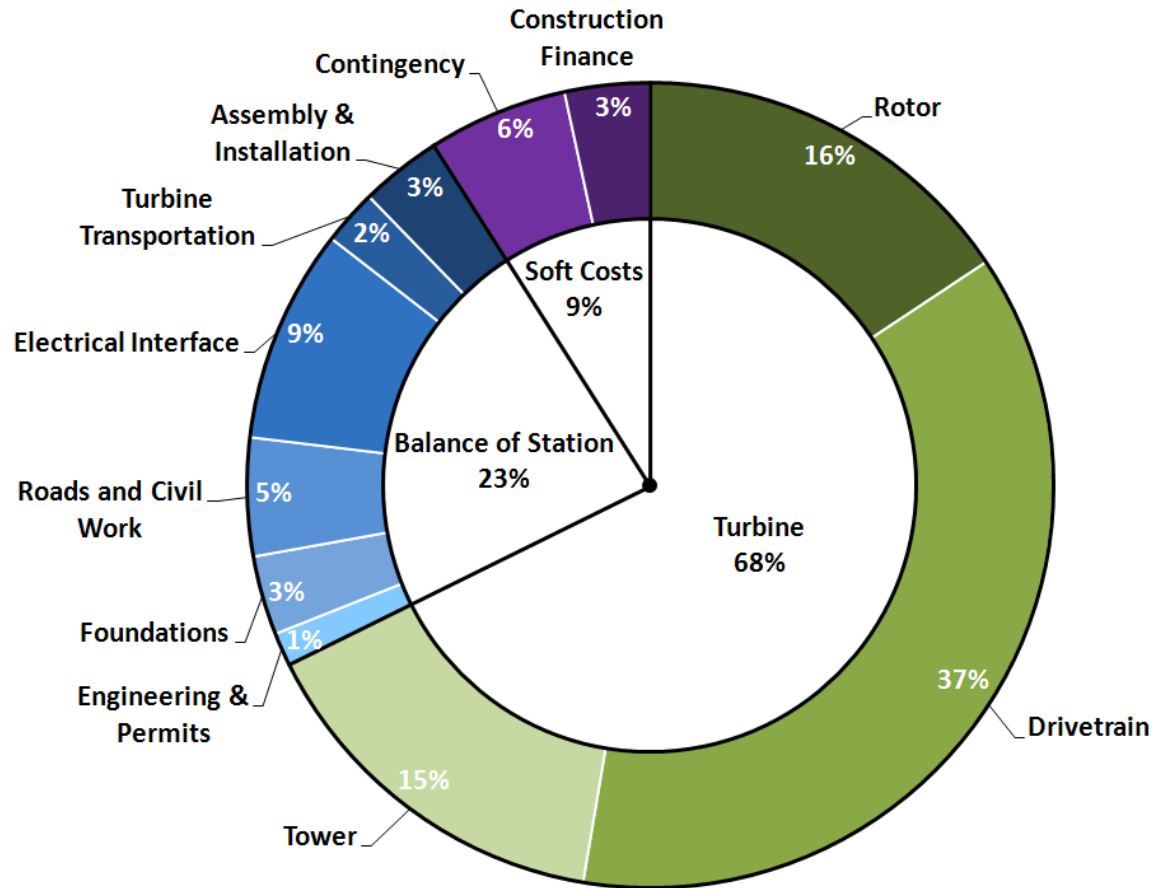
Source: Fraunhofer IWES



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-  Background
-  State of the Art
-  Problems and possibilities
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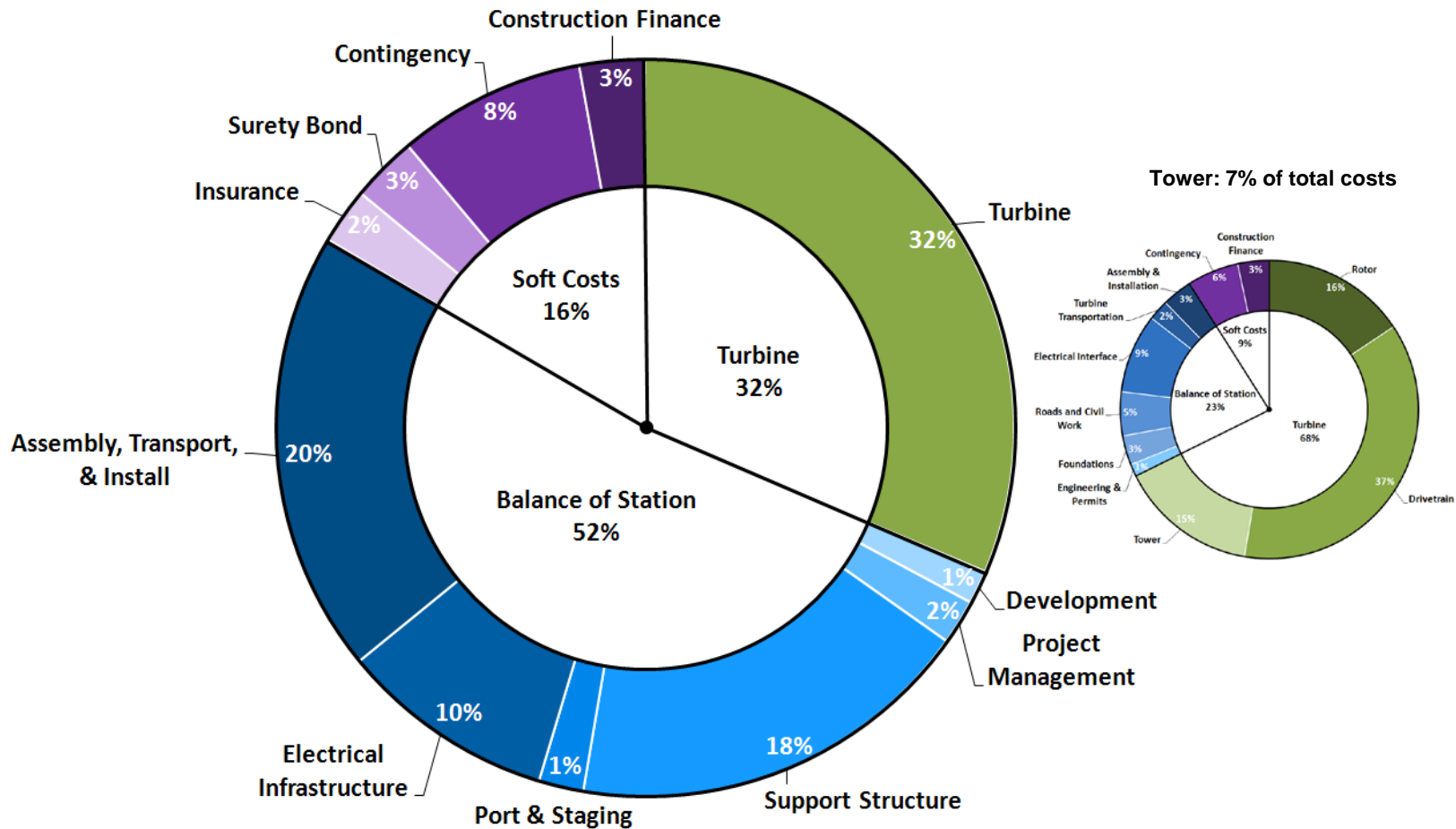
# Costs of tower and support structures Onshore: 15%



Source: NREL



# Costs of tower and support structures Offshore : 25%



Source: NREL

# State-of-the-art: towers of wind turbines

- < Wind turbines: amongst the largest and highest loaded structures
- < Due to growth of the industry and growth of installed capacity – repetition important
- < Also: the industry is more driven by innovation than the construction industry
- < Right now: S235, S355 are the main steel grades in use – as for the construction industry

# Overview



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State of the Art



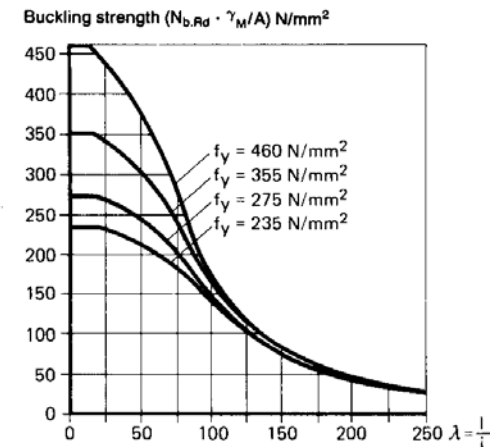
Problems and possibilities



Outlook

# Arguments in favour of use of higher steel grades

- > Static strength: increases linearly with  $f_y$
- > Buckling
  - > Slenderness:  $\lambda \approx l / D$  for thin-walled towers
  - > For low slenderness,  $\lambda \leq 50$  (e.g. 150 m high, 3 m  $\varnothing$ ), buckling strength increases almost linearly with the  $f_y$



# Arguments against use of higher steel grades

- < Weldability: more care needed
- < Toughness (earthquake resistance)
- < Fatigue
  - < With high SCF, no weld measures:  
virtually no influence of  $f_y$
  - < Lower SCF, post-weld treatment to introduce compressive stresses:  
almost linear increase with  $f_y$  achievable
- < Price
- < Lack of standards

# Price

- < Towers: dead weight no major cost post
- < Transport and installation costs is a factor
- < S235: 100%
- < S355: 103%, practically a no-brainer
- < S460: 110%, doable
- < S690: 170%, hardly economical over S460, unless weight is a severe problem

# Fatigue





- < Welding:
  - < Influence of the mean stress: typically unknown, a tensile mean stress equal to the yield stress has to be (conservatively) assumed, fatigue strength similar to lower grade steels, no benefit
  - < Unless this mean stress can be lessened or even be converted to a compressive stress, e.g. UIT (ultrasonic impact treatment)
- < Other connection methods, such as grouting or pre-stressed bolts can also help to utilize the higher potential strength, e.g. Siemens tower.

# Standards

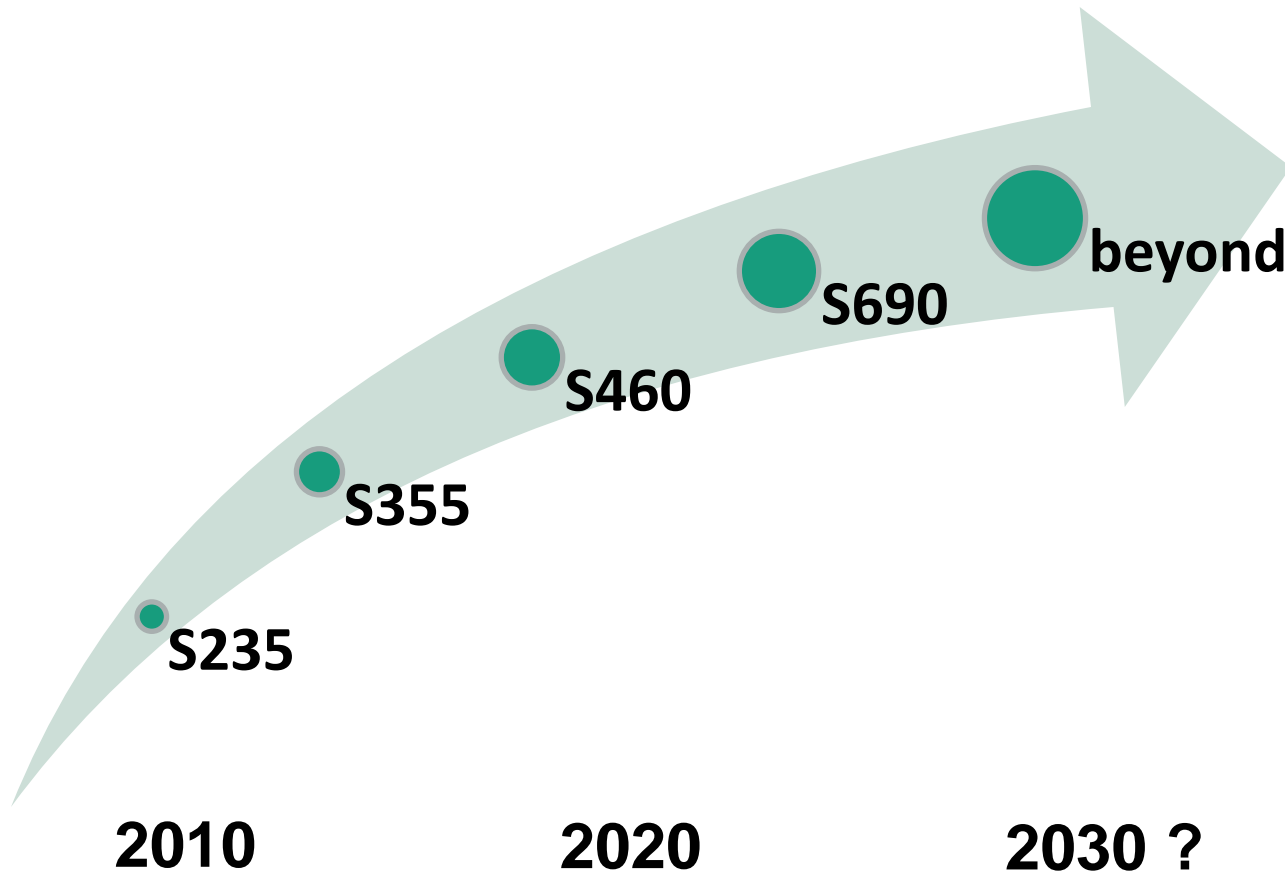
- ↪ EC 3: focus on mild steels, with no “bonus” for higher steel grades
- ↪ No bonus for fatigue improvement of post weld treatments
- ↪ Based on a rather rigid and simplistic classification of structural details
- ↪ The class  $\sigma_c$  is the stress range at 2 million cycles, S-N lines have a slope of 1:3 until 5 million cycles (at  $0.73 \sigma_c$ ) and a fatigue limit at  $0.40 \sigma_c$
- ↪ Can be used as a first, conservative approach
  
- ↪ GL: sceptical about use of steels exceeding S460:
- ↪ “high strength steels having nominal yield strengths (or 0.2% proof stresses) exceeding 460 N/mm<sup>2</sup> may be employed in exceptional cases only, with the corresponding technical justification and with GL consent
  
- ↪ Thus other standards need to be used (or developed!) in order to allow economical use of high strength steels for the fatigue loaded structures needed here



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# The road ahead



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# RAVE Offshore Wind R&D

International Conference on R&D for Offshore Wind Energy in the North Sea



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October 13-15, 2015  
Bremerhaven, Germany

Call for abstracts coming soon!



# THANK YOU FOR YOUR ATTENTION

Any questions?

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